

TORSION

1- Mechanics of Materials Approach

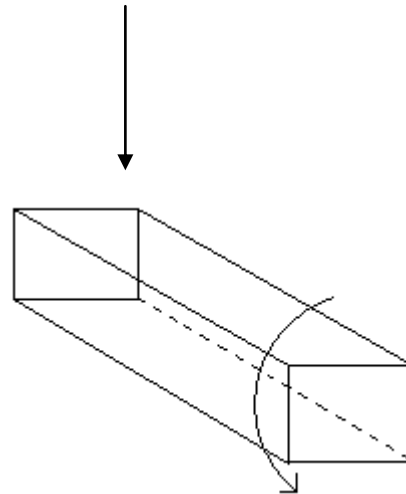
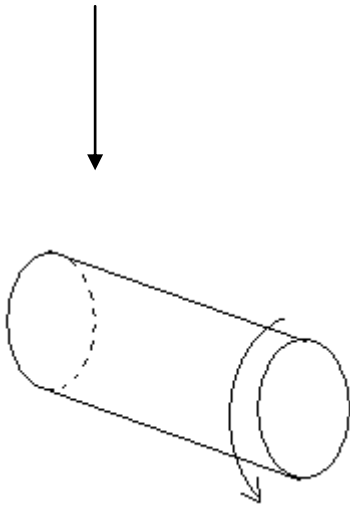
2- Prandtl Stress Function Approach

-more sufficient for axial cylinder problem

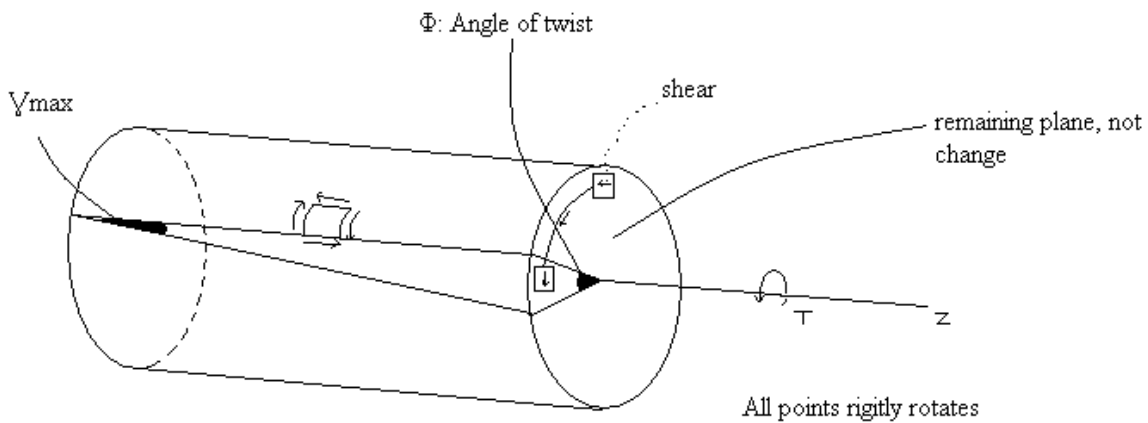
Sufficient

insufficient

sufficient



1-) Mechanics of Material Approach to Torsion of Circular Bars



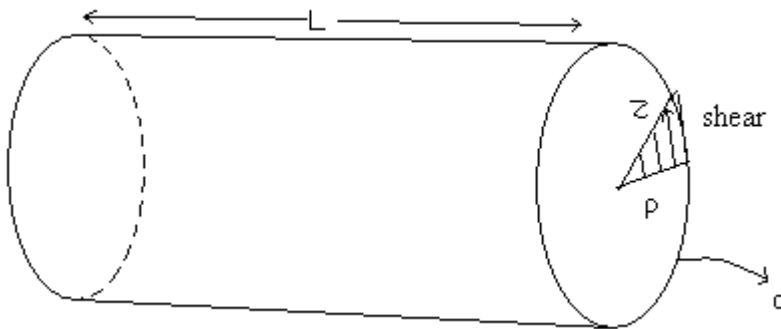
Assumptions:

1-) All plane sections perpendicular to torsion axis z remain perpendicular.

2-) Cross sections are undistorted in their individual planes

-Shearing strain γ varies linearly from at the center to a maximum at the outer surface

3-) Material is homogenous



$$T = \int \tau \cdot \rho dA$$

$$T = \int_0^r \frac{\rho}{r} \cdot \tau_{max} \cdot \rho dA = \frac{\tau_{max}}{r} \cdot \int \rho^2 dA$$

Where $\int \rho^2 dA$ is polar moment of inertia

$$J = \int \rho^2 dA \quad \text{and} \quad \tau_{max} = \frac{T \cdot r}{J} \quad \tau(\rho) = \frac{T \cdot \rho}{J}$$

Note: Polar moment of inertia for circle of radius r ; $J = \frac{\pi \cdot r^4}{2}$

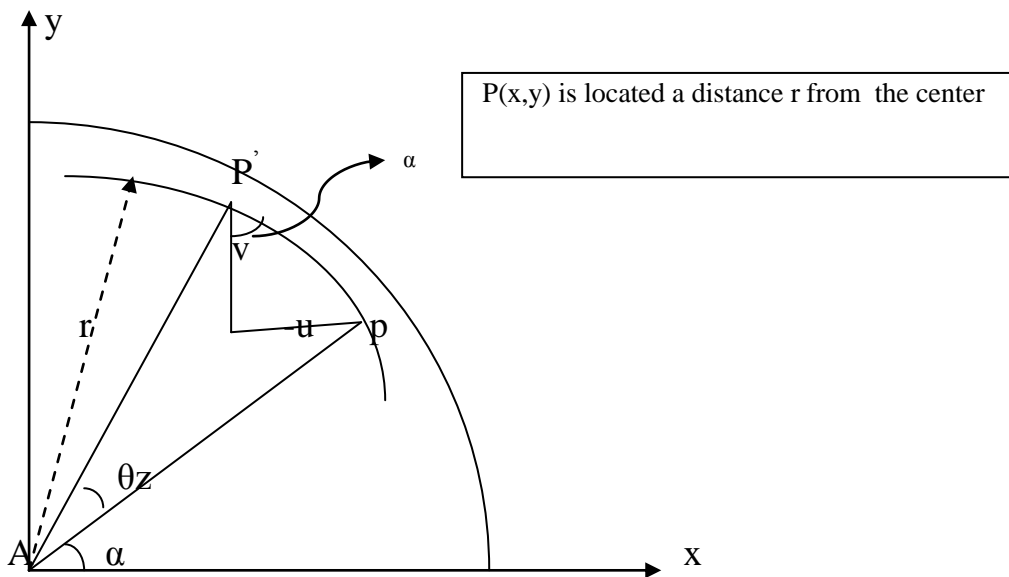
Angle of Twist Φ :

$$\Phi \cdot r = \gamma_{max} \cdot L$$

$$\gamma_{max} = \frac{\tau_{max}}{G} \quad (\text{Hooke's law}) \quad \text{so,} \quad \gamma_{max} = \frac{T \cdot r}{J \cdot G}$$

$$\Phi \cdot \underline{r} = \frac{T \cdot r}{J \cdot G} \cdot L \quad \Phi = \frac{T \cdot L}{J \cdot G} \quad J \cdot G : \text{Torsional rigidity}$$

2) theory of elasticity approach to torsion of bar



$$u = -(r\theta z)\sin\alpha = -y\theta z$$

$$v = (r\theta z)\cos\alpha = x\theta z$$

where the angular displacement of AP at a distance Z from the left end is θz

$$\epsilon_x = \gamma_{xy} = \epsilon_y = \epsilon_z = 0$$

$$\gamma_{zy} = \frac{\partial w}{\partial x} - y\theta, \quad \gamma_{zx} = \frac{\partial w}{\partial y} + x\theta$$

$$\sigma_x = \sigma_y = \sigma_z = \tau_{xy} = 0$$

$$\tau_{zx} = G\left(\frac{\partial w}{\partial x} - y\theta\right) \dots\dots\dots 1$$

$$\tau_{zy} = G\left(\frac{\partial w}{\partial y} + x\theta\right) \dots\dots\dots 2$$

assuming negligible body forces

$$\frac{\partial \tau_{zx}}{\partial z} = 0, \quad \frac{\partial \tau_{zy}}{\partial z} = 0, \quad \frac{\partial \tau_{zx}}{\partial x} + \frac{\partial \tau_{zy}}{\partial y} = 0$$

Differentiating the equation 1 and 2 w.r.t. y and x respectively, we have

$$\frac{\partial \tau_{zx}}{\partial y} - \frac{\partial \tau_{zy}}{\partial x} = H$$

Where

$$H = -2G\theta$$

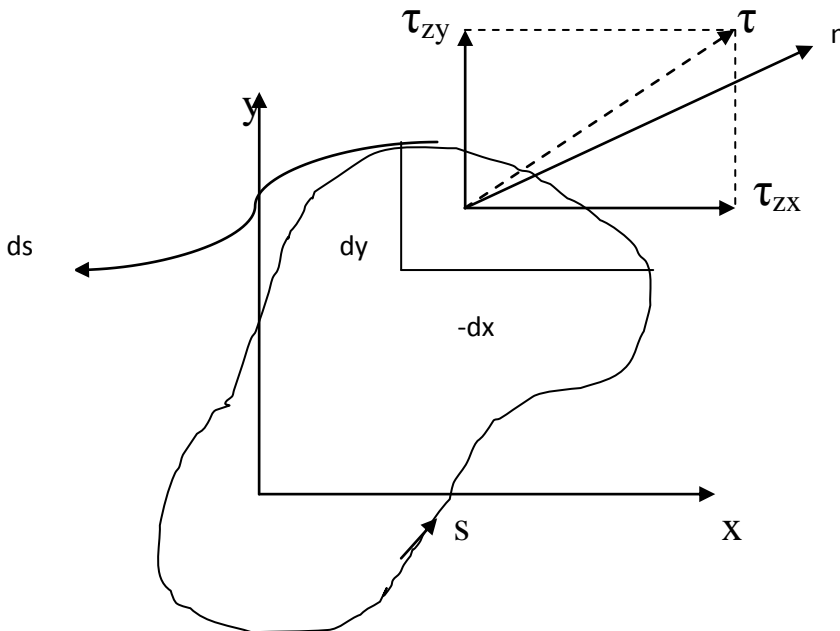
Stress function

As in the case of a beam the torsion problem formulated in the preceding is commonly solved by introducing a single stress function. If a function $\phi(x,y)$, the Prandtl stress function is assumed to exist, such that

$$\tau_{zx} = \frac{\partial \phi}{\partial y}, \quad \tau_{zy} = -\frac{\partial \phi}{\partial x}$$

$$\frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} = H$$

Boundary conditions



cosine of the angle between z and a unit normal n to the surface is zero that is $\cos(n,z) = 0$ we have

$$\tau_{zx}l + \tau_{zy}m = 0$$

τ must be tangent to the surface

$$l = \cos(n,x) = \frac{dy}{ds}, \quad m = \cos(n,y) = -\frac{dx}{ds}$$

so on the boundary we have

$$\frac{\partial \phi}{\partial y} \frac{dy}{ds} + \frac{\partial \phi}{\partial x} \frac{dx}{ds} = \frac{\partial \phi}{\partial s} = 0 \quad (\text{on the boundary})$$

References

Lecture notes

Ugural - Frenster